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Implementing AASHTO TP 110 for Alkali-Silica Reaction Potential Evaluation of Idaho Aggregates

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Abstract

The reaction between the active silica constituents of aggregates and alkalis in cement in the presence of moisture is called Alkali-Silica Reaction (ASR). ASR forms a swelling gel which can expand and cause internal stresses in cementitious materials leading to cracking, loss of strength, and eventually failure. The primary objective of this research study is to evaluate the advantages associated with implementing the new test method AASHTO TP-110 to better characterize the ASR potential of Idaho aggregates. Total of 8 identified aggregate types will be tested. The sources of those aggregate types are across Idaho with different degrees of ASR potential.



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1. INTRODUCTION
The Alkali-Silica reaction (ASR) is a destructive chem reaction that occurs between the active silica SiO ₂ constituents (reactive minerals) of aggregate and alkalis (Sodium-Na and Potassium-K) in the cement and other pozzolanic materials causing a definite expansion in the presence of moisture or a pore solution of concrete.
ASR forms a swelling gel, which can expand and cause internal stresses in cementitious materials leading to cra loss of strength, and eventually failure of concrete or con structures.
 Three essential conditions are necessary to create ASR induced damage in concrete structures: Presence of reactive siliceous components in aggreg Sufficient availability of OH⁻ ions and alkalis (Na⁺ and - Sufficient moisture (above 75% RH).
2. OBJECTIVE
Evaluate advantages (as compared to other test method associated with implementing AASHTO TP-110, a new te method to evaluate aggregate susceptibility to ASR withi specifications to better characterize the ASR potential of aggregates.
The baseline ASR susceptibility for Idaho aggregates will established. ASR potentials quantified through the AASH TP-110 procedure (MCPT) will be evaluated in light of A C1293 and ASTM C 1260 (AASHTO T 303) test results.
3. ASR IN IDAHO AGGREGATES
Results from Idaho Transportation Department (ITD) res project RP 212 confirmed 80% of the aggregates used in Idaho are reactive, or highly reactive.
The primary bases for determining the reactivity of Idaho aggregate are still ASSHTO T 303 or ASTM C 1260 and C 1293.
According to RP 212, a very limited number of Idaho aggregate sources passed the ASTM C1260 test. Means several aggregates that failed in ASTM C1260 passed th one-year ASTM C1293 test (Gillerman and Weppner, 20)
ASTM C 1260 gives false negative and false positive results for different aggregate samples of Idaho.
<u>References:</u> 1. AASHTO, TP 110-14, 2014, "Standard Method of Test for Potential Alkali Reactivity of Aggregates and

Presenter : Md Shahjalal Chowdhury, Research Assistant



Effectiveness of ASR Mitigation Measures (Miniature Concrete Prism Test, MCPT)". American Association of State Highway and Transportation Officials (AASHTO), Washington, DC, USA. https://www.transportation.org/ Gillerman V. S., and Weppner K. N. (2014), Lithologic Characterization of Active ITD Aggregate Sources and Implications for Aggregate, Idaho Geological Survey, University of Idaho, Moscow, Idaho Latifee, E. R., & Rangaraju, P. R. (2014). Miniature concrete prism test: rapid test method for evaluating alkali-silica reactivity of aggregates. Journal of Materials in Civil Engineering, 27(7), 04014215.

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DLVED	IN ASR	6.CC	RRE	LAT	
ds the	Map Cracking in Concrete	CPI			
ncrete		 During the development tested (19 coarse aggreent tested) Correlation of MCPT with results found for the set to the set of the s			
1					
			%Expansion		
ы. С				ASTM	ASTM C 1260
ca Gol		Aggregate Identity	MCPT 56 days (CV %)	(ASTM 2007a) 365 days	(ASTM 2007b) 14 days
		SP	0.149 (4.08)	0.181	0.350
ALKALI	Once ASR damage has	SD	0.099 (4.97)	0.109	0.220
Na+	begun, expansion		0.185 (3.43)	0.251	0.900
114	continues which leads to	BB	0.017 (8.81)	0.032	0.042
FORCE OH-	further cracking	GLN	0.046 (4.34)	0.050	0.235
0.		QP	0.070 (3.01)	0.070	0.080
Na+ K+ SiO-		SLC	0.039 (8.31)	0.030	0.190
DH-		MSP	0.023 (2.47)	0.030	0.100
Na+		GI	0.440 (4.21)	0.590	0.840
		SB	0.115 (9.83)	0.150	0.460
HOUS OH-	leads to increased permeability of the	Pi Agg	roposed (gregate R	Criteria f eactivity	or Charac in the M0
a Gel	concrete	Degree of	Reactivity	Expansion at Days, %	56 A O
1₂O)		Non-re	active	(8 Weeks) < 0.030	
2 - 1		Non-re	active	0.031-0.040) <
OH-		Low/Slow	reactive	0.031-0.040) >
ALKALI IONS		Moderatel	y reactive	0.041-0.120)
+	Ingress of water into the	Highly r	eactive	0.121-0.240)
OH- H ₂ O	concrete increases the	very mgm	yreactive	20.240	
Sio- K+	therefore the ASR				
O-Na		7.10) E N T	'I F I I	ED A
H20 SIO.				aaroa	ato mot
.0-				iggrega	
он-		fine p	parts wi	ll be te	sted for
REACTION WITH OH		set-u	ips are	operati	ng in ol
		reau	' ired Th	' e eyne	oted teg
MTE	ST (MCPT)	Icqu Ast		c crpc	
		1 ³ , 4	2019.		
Test (MC	PT) was developed at				
an altern	ative to the existing	Seve	eral AME	BT (AA	SHTO 1
		to ide	antify th	o non i	reactive
	293 to evaluate				
		AASHTO T-303 Test Results of Poforonco Aggrogatos			
		Reference Aggregates			
st Temne	rature	Name	Expansion 16 Davs.	on at Rea (%)	ctivity (max ex allowed 0.10
. 1 7°C (1	$10 \pm 3^{\circ}$ E)	Wn 56	0.616	5	Reactive
		Dolomi	te 0.187	7	Reactive
		Quart	z 0.198	3 I	Reactive
		Granite	L1 0.075	5	Non-reactive
		Granite	L2 0.009)	Non-reactiv
active: If	expansion				
030% at 5	6 days	8. S	UMM	ARY	
	rnansion				
$\Lambda \Lambda 0/$ of 5	6 dave	Implementing the MCPT to			
140 /0 al J	o days.	of A	SR read	ctivity v	vill help
				-	•



OF MCPT WITH AMBT AND

nt of MCPT, a total 33 aggregates sample were egate and 14 fine aggregates).

ith AMBT and CPT are developed based on the elected aggregates.



GGREGATE MATERIALS

terials have been identified. Both coarse and each aggregate type. Currently, twelve test ur lab to expedite the total testing time st completion date of our first aggregate is July

T-303) tests were run on different aggregates reference aggregate types

AASHTO T-303 Test Results of Identified Aggregates Sample Expansion at Reactivity (max expansion allowed 0.10%) 16 Days, (%) 0.50-0.59 Reactive EL 116c 0.50-0.59 Reactive ORE 8c 0.50-0.59 Reactive MD 450 0.10-0.19 BG 111 Reactive 0.30-0.39 Reactive **BN 155** >0.70 Ma 220 Reactive 0.40-0.49 Reactive LN 80c WN 56c 0.61 Reactive

test method into Idaho practice and mitigation increase the longevity of concrete structures.